

# SPREAD FOOTING SECTION

# NOTE

At (a) and Short (a) bars: H < 1800 mm, no splices are allowed within 500 mm above the top of footing.  ${\rm H}$  > 1800 mm, no splices are allowed within  ${\rm H}/{\rm 4}$ 

above the top of footing.

Number above short(d) bars indicates distance H=3600 from top of footing to upper end of short(a) bars. H=3000 (a) Bars Short (a) bars 1100 H=1800 1000 H=1200

125-

200 150-

**ELEVATION** 

CASE III CASE IV DETAIL OF DESIGN LOADING CASES

Case I Level +II.5 kPa surcharge Case Ⅲ 1:2 Unlimited slope Case III 1:1.5 Limited slope (1500 max height)

Case IV 1:1.5 Unlimited slope

Level + II.5 kPa

surcharge

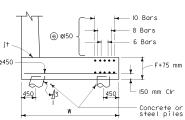
### MAX PILE SPACING FOR 400 KN PILES

FUR 400 KN PILES								
Design	Front Row	Back Row						
H	1:3 Batter	Vertical						
1200	5400	5400						
1800	3600	5400						
2400	2700	5400						
3000	1800	3600						
3600	1200	2400						

For actual spacing,

see Wall Layout.

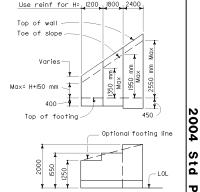
Pile layout does not apply to Case Ⅳ conditions.



Reinforcement detailed is to be placed in addition to that shown for spread footings.

● For Design H=1200 use W=1550 All others from table

# Owncomer L Nor vercomer Y. H July 1, 2004 45803 p. 12-31-06 get to the Caltrans web site, go to: http://www.dot.ca.gov



TYPICAL LAYOUT EXAMPLE

# 400 kN PILE FOOTING SECTION

# NOTES

### Design Conditions:

Design H may be exceeded by I50 mm before going to the next size. Special footing design is required where foundation material is incapable of supporting toe pressure loads listed in table.

## Design Data:

f<sub>C</sub> = 10 MPa  $f_C' = 25 \text{ MPa}$   $f_S = 168 \text{ MPa}$   $n = 10 \text{ earth} = 19 \text{ kN/m}^3$ Case I- Wall design for equivalent fluid pressure = 4.2 and 5.6 kPa/m. Case II, III, IV - Wall design is based on Rankine's formula with  $\phi = 33^{\circ} 42'$ .

TABLE OF REINFO	ORCING	STEEL,	DIMENS	IONS ANI	DATA
Design H	<ul><li>1200</li></ul>	1800	2400	3000	3600
W	1250	1550	2000	2450	2900
F Spread Ftg	400	400	450	450	550
Batter	None	None	None	100:3	100:6
(a) Bars	#16@400	#16@400	#16@400	#I6@300	*I6@250
Short (a) Bars	None	None	#I6@400	<b>■</b> 16@300	#I6@250
(b) Bars	*I6@400	#16@400	#16@200	#16@150	#16@125
Total (e) Bars	8-#19	8-#19	10-#19	8-=19	6-#19
O Case I kPa	80	105	120	145	170
೪ೄ Case Ⅲ kPa	75	100	130	165	195
⊢ % Case III kPa	80	110	140	185	210
à Case IV kPa	95	155	200	255	310

STATE OF CALIFORNIA DEPARTMENT OF TRANSPORTATION

For joints required, see

# RETAINING WALL TYPE 5

NO SCALE

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS OTHERWISE SHOWN B3-7

# -II.5 kPa surcharge

+II.5 kPa surcharge

ΑN

8

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